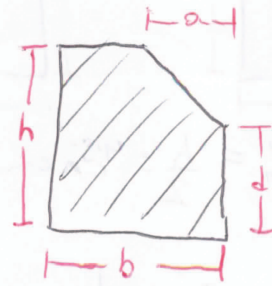
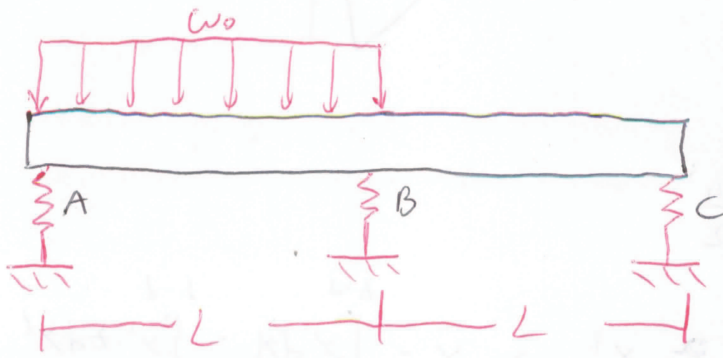


P1)



Esquema general del problema:

Determinar DCL  $\Rightarrow$  No pueden encontrarse todas las reacciones (Hiperestático)

Para geometrias complejas, hace falta calcular  $I_z$

Ecua. de la elástica  $\frac{d^4 \hat{y}}{dx^4} = - \frac{w(x)}{EI_z}$

Calcular ej. neutro de la geometría

$\Rightarrow$  Calcular  $I_z$

$\Rightarrow$  Encontrar  $\hat{y}(x)$  con sus cond. de borde

$\Downarrow$   
Encontrar reacciones

(I) Cálculo de propiedades de área ( $I_z$ )



$$\bar{y} = \frac{\int y' dA}{A}$$

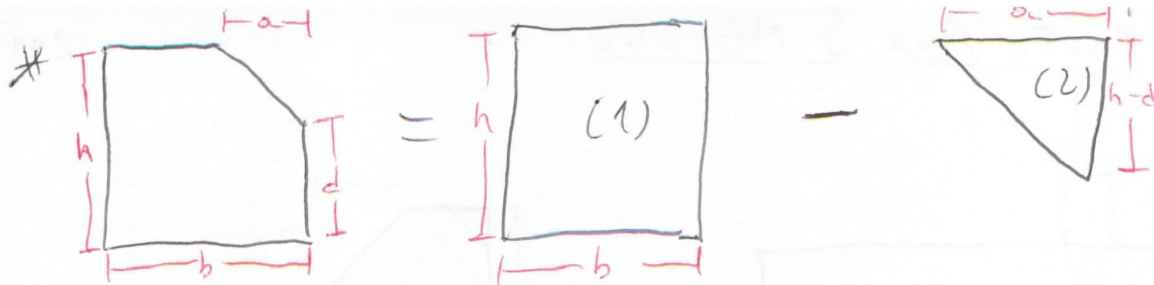
$$I = \int y'^2 dA = \int (y' - \bar{y})^2 dA$$

• Si hay varias fig. geométricas:

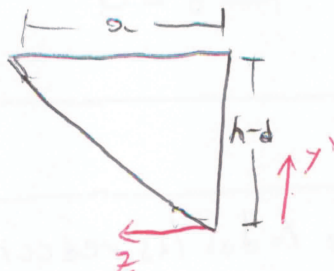
$$\bar{y}_T = \frac{\sum \bar{y}_n \cdot A_n}{\sum A_n}$$

• Traslado de eje de rotación

$$\bar{I}_z = I_z + s^2 \cdot A$$



• Para (1):  $\bar{y}_1 = \frac{h}{2} = 4,5 \text{ cm} \mid I_{z1} = \frac{bh^3}{12}$

• Para (2):   $z = \frac{a}{h-d} y' \Rightarrow \bar{y}_2 = \int_0^{h-d} y' dA = \int_0^{h-d} y' \cdot z dy' = \frac{\int_0^{h-d} y'^2 \frac{a}{h-d} dy'}{a(h-d)} \cdot 2$

$$\therefore \bar{y}_2 = \frac{2}{3}(h-d) = 4 \text{ cm}$$

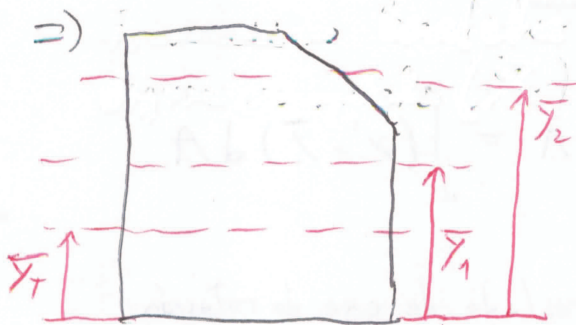
$$\Rightarrow I_{zz} = \int y^2 dA = \int (y' - \bar{y}_2)^2 dA = \int (y'^2 - 2y'\bar{y}_2 + \bar{y}_2^2) \frac{a}{h-d} y' dy' = \frac{a}{h-d} \left( \frac{(h-d)^4}{4} - 2 \cdot \frac{2}{3}(h-d) \cdot \frac{(h-d)^3}{3} + \left( \frac{2}{3}(h-d) \right)^2 \cdot \frac{(h-d)^2}{2} \right)$$

$$I_{zz} = \frac{a}{36} (h-d)^3$$

$$\Rightarrow \bar{y}_T = \frac{\bar{y}_1 \cdot A_1 - \bar{y}_2 \cdot A_2}{A_1 - A_2} = \frac{\frac{h}{2} \cdot bh - \left( \frac{2}{3}(h-d) + d \right) \cdot \frac{a(h-d)}{2}}{b \cdot h - \frac{a(h-d)}{2}}$$

Desde la base!

$$\bar{y}_T = 3,8421 \text{ cm}$$



• Es necesario encontrar el momento de inercia de cada sección con respecto al eje neutro total ( $\bar{y}_T$ )

$$\Rightarrow I_{z1} = I_{z1} + (\bar{y}_1 - \bar{y}_T)^2 \cdot A_1 = \frac{bh^3}{12} + \left( \frac{h}{2} - \bar{y}_T \right)^2 \cdot b \cdot h$$

$$I_{z1} = 517,164 \text{ cm}^4$$

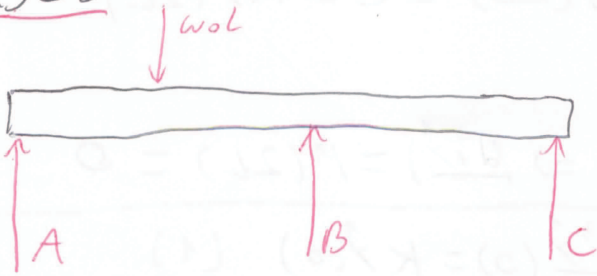


$$\Rightarrow I_{z2} = I_{z1} + (Y_2 - Y_1) \cdot A_2 = \frac{\pi}{36}(h-d)^3 + \left(\frac{2}{3}(h-d) + d - Y_1\right) \cdot \frac{\pi(h-d)}{2}$$

$$\overline{I}_{z2} = 179,885 \text{ cm}^4$$

$$\begin{aligned} \therefore I_T &= \overline{I}_{z1} - \overline{I}_{z2} = 337,879 \text{ cm}^4 \\ &= 3,3788 \cdot 10^{-6} \text{ m}^4 \end{aligned}$$

(II) DCL



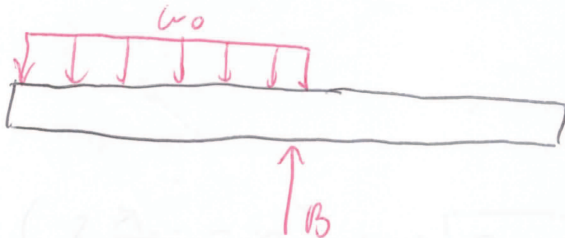
$$\sum F_y = 0 \Rightarrow A + B + C = w_0 L$$

$$\sum M_z = 0 \Rightarrow -\frac{w_0 L^2}{2} + BL + 2CL = 0$$

$$\therefore C = \frac{1}{2}(w_0 \frac{L}{2} - B) \wedge A = \frac{1}{2}(\frac{3}{2}w_0 L - B)$$

(III) Deflexión: Modelación de fuerzas mediante función escalón (Fzas. distribuidas) o delta de Dirac (Fzas puntuales)

Lo que ocurre en dos bordes se reemplaza por condiciones de Borde. Son necesarias 4.



$$\Rightarrow w(x) = w_0(r(x) - r(x-L)) + B \cdot \delta(x-L)$$

$$\leadsto \frac{d^4 y}{dx^4} = -\frac{w(x)}{EI_z} = -\frac{1}{EI_z} (w_0(r(x) - r(x-L)) - B \cdot \delta(x-L))$$

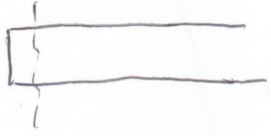

$$\leadsto \frac{d^3 y}{dx^3} = -\frac{1}{EI_z} (w_0(x r(x) - (x-L)r(x-L)) - B \cdot r(x-L)) + \alpha$$

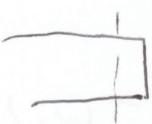
$$\leadsto \frac{d^2 y}{dx^2} = -\frac{1}{EI_z} (w_0(\frac{x^2}{2} r(x) - \frac{(x-L)^2}{2} r(x-L)) - (x-L)B r(x-L)) + 2x + B$$

$$\leadsto \frac{dy}{dx} = -\frac{1}{EI_z} (w_0(\frac{x^3}{6} r(x) - \frac{(x-L)^3}{6} r(x-L)) - \frac{(x-L)^2}{2} B r(x-L)) + \frac{\alpha x^2}{2} + Bx + y$$

$$\rightarrow \hat{y} = -\frac{1}{EIz} \left( \omega_0 \left( \frac{x^4}{24} r(x) - \frac{(x-L)^4}{24} r(x-L) \right) - \frac{(x-L)^3}{6} B r(x-L) \right) + \frac{2x^3}{6} + \frac{\beta x^2}{2} + \gamma x + \delta$$

#### (IV) Condiciones de Borde

(A)   $\Rightarrow$    $\Rightarrow V(0) = -A = -K \hat{y}(0)$

(C)   $\Rightarrow$    $\Rightarrow V(2L) = C = K \hat{y}(2L)$

\* No hay empotramientos en los bordes  $\Rightarrow M(0) = M(2L) = 0$

$\therefore$  Condiciones de Borde:  $-EI \frac{d^3 \hat{y}}{dx^3}(0) = K \hat{y}(0)$  (1)

$-EI \frac{d^3 \hat{y}}{dx^3}(2L) = -K \hat{y}(2L)$  (2)

$\frac{d^2 \hat{y}}{dx^2}(0) = 0$  (3)  $\wedge$   $\frac{d^2 \hat{y}}{dx^2}(2L) = 0$  (4)

\* Ahora el problema puede resolverse:

(3):  $\frac{d^2 \hat{y}}{dx^2}(0) = 0 \Rightarrow \boxed{\beta = 0}$

(1):  $-EI \frac{d^3 \hat{y}}{dx^3}(0) = K \hat{y}(0) \Rightarrow \boxed{-EI \alpha = K \cdot \delta}$  (i)

(4):  $\frac{d^2 \hat{y}}{dx^2}(2L) = 0 \Rightarrow -\frac{1}{EI} \left( \omega_0 \cdot 2L^2 - \omega_0 \frac{(2L-L)^2}{2} - LB \right) + 2L\alpha = 0$

$\alpha = \frac{1}{2EI} \left( \omega_0 \cdot \frac{3}{2} L^2 - LB \right)$

$\alpha = 1,17 \cdot 10^{-3} - 7,8 \cdot 10^{-7} \beta$

$\delta = -\frac{EI \alpha}{K} = -7,5 \cdot 10^{-4} + 5 \cdot 10^{-7} \beta$



$$(4) \cdot EI \frac{d^4 y}{dx^4}(2L) = K y(2L)$$

$$\rightarrow \omega_0(2L-L) + B + \alpha = -\frac{K}{EI} \left( \frac{\omega_0}{24} ((2L)^4 - L^4) - \frac{L^3}{6} B \right) + \left( \frac{\alpha (2L)^3}{6} + \underline{\underline{\gamma \cdot 2L}} + \delta \right) K$$

$$\omega_0 L + B + \alpha = -\frac{K}{EI} \left( 15L^4 - \frac{L^3}{6} B \right) + \frac{4}{3} \alpha L^3 + 2L\gamma + \delta$$

$$\Rightarrow \gamma = -4,23284 \cdot 10^{-5} + 3,87771 \cdot 10^{-7} B$$

\* Finalmente, con las constantes calculadas, podemos encontrar la fuerza B, sabiendo que:  $B = -K y(L)$

$$\Rightarrow B = \frac{K}{EI} \omega_0 \frac{L^4}{24} + \left( \frac{\alpha L^3}{6} + \gamma L + \delta \right) K$$

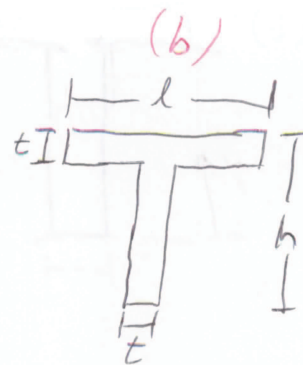
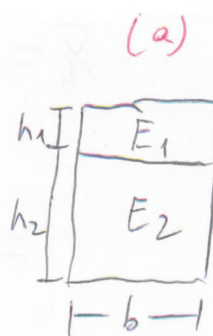
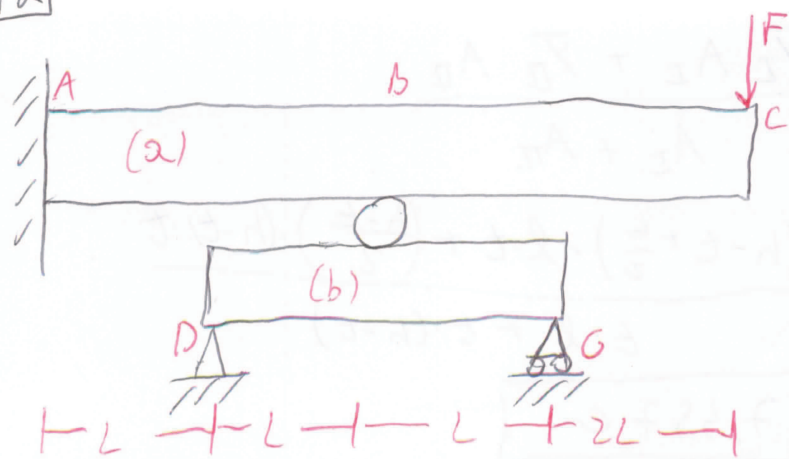
$$\therefore B = 376,4 \text{ N}$$

\* Reemplazando en el DCL, se tiene finalmente que

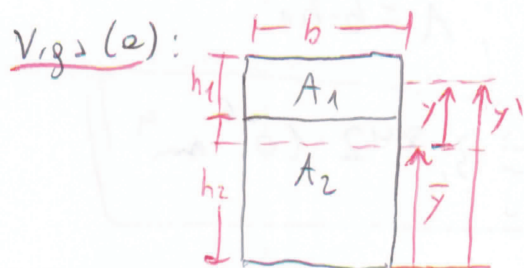
$$C = 61,8 \text{ [N]} \quad \wedge \quad A = 561,801 \text{ [N]}$$



Pa



a) Cálculo de ejes neutros:



$$E_1 \int_{A_1} y' dA + E_2 \int_{A_2} y' dA = 0$$

$$\begin{aligned} \Rightarrow E_1 \int_{A_1} y' dA + E_2 \int_{A_2} y' dA &= E_1 \int_{A_1} (\bar{y} + y) dA + E_2 \int_{A_2} (\bar{y} + y) dA \\ &= E_1 \int_{A_1} \bar{y} dA + E_2 \int_{A_2} \bar{y} dA + \left( E_1 \int_{A_1} y dA + E_2 \int_{A_2} y dA \right) \\ &= E_1 \bar{y} A_1 + E_2 \bar{y} A_2 \end{aligned}$$

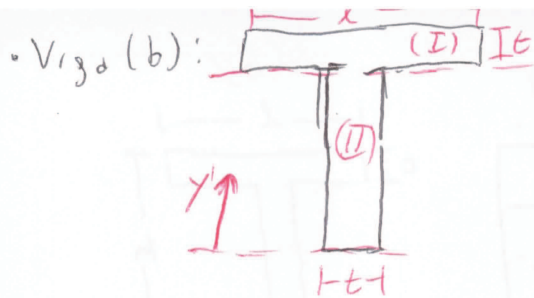
$$\therefore \bar{y}_a = \frac{E_1 \int_{A_1} y' dA + E_2 \int_{A_2} y' dA}{E_1 A_1 + E_2 A_2}$$

$$\Rightarrow \int_{A_1} y' dA = \int_{h_2}^{h_2+h_1} y' \cdot b dy' = \frac{b}{2} ((h_2+h_1)^2 - h_2^2) = \frac{bh_1}{2} (2h_2+h_1) = 384 \text{ cm}^3$$

$$\int_{A_2} y' dA = \int_0^{h_2} y' \cdot b dy' = \frac{bh_2^2}{2} = 400 \text{ cm}^3$$

$$\therefore \bar{y}_a = 8,733 \text{ cm}$$





$$\bar{Y}_b = \frac{\bar{Y}_I \cdot A_I + \bar{Y}_{II} \cdot A_{II}}{A_I + A_{II}}$$

$$= \frac{(h-t+\frac{t}{2}) \cdot l \cdot t + (\frac{h-t}{2}) \cdot (h-t) \cdot t}{l \cdot l + t \cdot (h-t)}$$

$$\boxed{\bar{Y}_b = 7,557 \text{ cm}}$$

b) Cálculo de momentos de área: Usando teorema de ejes paralelos

• Viga (a): •  $I_1 = \frac{b \cdot h_1^3}{12}$  ;  $\delta^2 = (h_2 + \frac{h_1}{2} - \bar{Y}_a)^2$  ;  $A = b \cdot h_1$

$$\therefore \bar{I}_1 = \frac{b \cdot h_1^3}{12} + (h_2 + \frac{h_1}{2} - \bar{Y}_a)^2 \cdot A = 3,842 \cdot 10^{-6} \text{ m}^4$$

•  $I_2 = \frac{b \cdot h_2^3}{12}$  ;  $\delta^2 = (\frac{h_2}{2} - \bar{Y}_a)^2$  ;  $A = b \cdot h_2$

$$\therefore \bar{I}_2 = \frac{b \cdot h_2^3}{12} + (\frac{h_2}{2} - \bar{Y}_a)^2 \cdot b \cdot h_2 = 1,781 \cdot 10^{-5} \text{ m}^4$$

• Viga (b):  $\bar{I}_b = \bar{I}_{(I)} + \bar{I}_{(II)}$

$$\rightarrow I_{(I)} = \frac{l t^3}{12} ; \delta^2 = (h-t+\frac{t}{2} - \bar{Y}_b)^2 ; A = l t$$

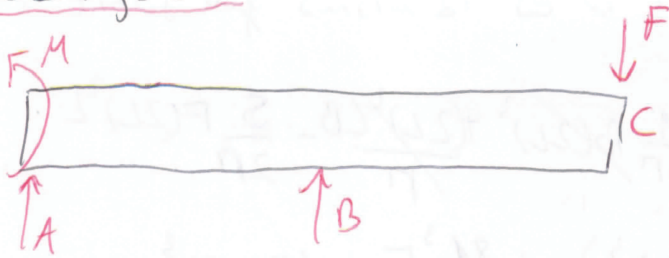
$$\Rightarrow \bar{I}_{(I)} = \frac{l t^3}{12} + (h-t+\frac{t}{2} - \bar{Y}_b)^2 \cdot l t = 56,109 \text{ cm}^4$$

$$\rightarrow I_{(II)} = \frac{t \cdot (h-t)^3}{12} ; \delta^2 = (\frac{h-t}{2} - \bar{Y}_b)^2 ; A = t(h-t)$$

$$\Rightarrow \bar{I}_{(II)} = \frac{t(h-t)^3}{12} + (\frac{h-t}{2} - \bar{Y}_b)^2 \cdot t(h-t) = 134,212 \text{ cm}^4$$

$$\therefore \bar{I}_b = 190,321 \text{ cm}^4 = 1,903 \cdot 10^{-6} \text{ m}^4$$

c) VCL viga (a)



$$\sum F_y = 0 \Rightarrow A + B = F$$

$$\sum M_A = 0 \Rightarrow M + 2LB - 2LF = 0$$

Deflexión:  $w(x) = B \delta(x-2L)$  y  $I = E_1 I_1 + E_2 I_2$

$$\rightarrow \frac{d^4 y}{dx^4} = \frac{B}{I} \delta(x-2L)$$

$$\rightarrow \frac{d^3 y}{dx^3} = \frac{B}{I} \delta(x-2L) + \alpha$$

$$\rightarrow \frac{d^2 y}{dx^2} = \frac{B}{I} (x-2L) \delta(x-2L) + \alpha x + \beta$$

$$\rightarrow \frac{dy}{dx} = \frac{B}{2I} (x-2L)^2 \delta(x-2L) + \frac{\alpha x^2}{2} + \beta x + \gamma$$

$$\rightarrow y(x) = \frac{B}{6I} (x-2L)^3 \delta(x-2L) + \frac{\alpha x^3}{6} + \frac{\beta x^2}{2} + \gamma x + \delta$$

Condiciones de borde

[A]  $\begin{cases} y(0) = 0 & (1) \\ \frac{dy}{dx}(0) = 0 & (2) \end{cases}$

[C]  $\frac{d^2 y}{dx^2}(2L) = 0 \quad (3)$

$\begin{matrix} \uparrow & \downarrow & \downarrow F \\ \square & \square & \end{matrix} \Rightarrow V(2L) = -F$

$\therefore \frac{d^3 y}{dx^3}(2L) = \frac{F}{I} \quad (4)$

$\Rightarrow (1): \delta = 0$

$(2): \gamma = 0$

$(3): \frac{3BL}{I} + \alpha L + \beta = 0 \quad (*)$

$(4): \frac{B}{I} + \alpha = \frac{F}{I} \Rightarrow \alpha = \frac{1}{I} (F - B)$

$\Rightarrow E_n(*): \beta = -\left(\frac{3BL}{I} + \frac{SL}{I} (F - B) + \frac{SL}{I} B\right) \Rightarrow \beta = \frac{2BL}{I} - \frac{SFL}{I}$

\* La deflexión que sufre la viga (a) en B es la misma que sufre (b) en el mismo punto. Luego:

$$\begin{aligned} \text{Viga (a): } \delta_a(2L) &= \frac{1}{6\Gamma} F(2L)^3 - \frac{1}{6\Gamma} B(2L)^3 + \frac{(2L)^2 LB}{\Gamma} - \frac{5}{2\Gamma} F(2L)^2 L \\ &= B \left( \frac{4L^3}{\Gamma} - \frac{4L^3}{3\Gamma} \right) + \frac{8L^3}{6\Gamma} F - \frac{10}{\Gamma} FL^3 \end{aligned}$$

$$\delta_a(2L) = \frac{8L^3}{3\Gamma} B - \frac{22L^3}{3\Gamma} F$$

$$\text{Viga (b): } \delta_b(L) = \frac{-BL^3}{6EI_b}$$

$$\Rightarrow \delta_a(2L) = \delta_b(L) \leadsto \frac{8L^3}{3\Gamma} B - \frac{22L^3}{3\Gamma} F = \frac{-BL^3}{6EI_b}$$

$$\frac{8B}{\Gamma} + \frac{B}{2EI_b} = \frac{22}{\Gamma} F$$

$$\therefore B = \frac{\frac{22}{\Gamma} \cdot F}{\frac{8}{\Gamma} + \frac{1}{2EI_b}} = 2410,355 \text{ N}$$

Volviendo a DCL:

$$A = F - B = -1410,355 \text{ N}$$

$$M = 5LF - 2LB = 179,29 \text{ N}\cdot\text{m}$$