

Pauta Ejercicio 3 CI52K Primavera 2009

Datos geométricos de la viga

$$\begin{aligned}h &:= 1220 \cdot \text{mm} & h_{ft} &:= 150 \cdot \text{mm} \\bw &:= 355 \cdot \text{mm} & \bar{L} &:= 24 \cdot \text{m} \\b &:= 1780 \cdot \text{mm}\end{aligned}$$

Propiedades geométricas:

$$\begin{aligned}A_c &:= b \cdot h_{ft} + (h - h_{ft}) \cdot bw & A_c &= 6.469 \times 10^5 \cdot \text{mm}^2 \\y_b &:= \frac{b \cdot h_{ft} \cdot \left(h - \frac{h_{ft}}{2}\right) + (h - h_{ft}) \cdot bw \cdot \frac{(h - h_{ft})}{2}}{A_c} & y_b &= 786.789 \cdot \text{mm} \\y_t &:= h - y_b & y_t &= 433.211 \cdot \text{mm} \\I_c &:= \frac{b \cdot h_{ft}^3}{12} + b \cdot h_{ft} \cdot \left(h - \frac{h_{ft}}{2} - y_b\right)^2 + \frac{bw \cdot (h - h_{ft})^3}{12} + (h - h_{ft}) \cdot bw \cdot \left(\frac{h - h_{ft}}{2} - y_b\right)^2 \\I_c &= 9.508 \times 10^{10} \cdot \text{mm}^4 \\r &:= \sqrt{\frac{I_c}{A_c}} & r &= 383.398 \cdot \text{mm} \\W_t &:= \frac{I_c}{y_t} & W_t &= 2.195 \times 10^8 \cdot \text{mm}^3 \\W_b &:= \frac{I_c}{y_b} & W_b &= 1.208 \times 10^8 \cdot \text{mm}^3 \\c1 &:= y_t & S1 &:= W_t \\c2 &:= y_b & S2 &:= W_b\end{aligned}$$

Cargas:

$$\begin{aligned}\gamma_c &:= 25 \frac{\text{kN}}{\text{m}^3} & q_o &= 16.171 \frac{\text{kN}}{\text{m}} & M_o &:= \frac{q_o \cdot L^2}{8} \\q_o &:= A_c \cdot \gamma_c & & & M_d &:= \frac{q_d \cdot L^2}{8} \\q_d &:= 5.8 \frac{\text{kN}}{\text{m}} & & & & \end{aligned}$$

$$q_l := 17.5 \frac{\text{kN}}{\text{m}}$$

$$M_l := \frac{q_l \cdot L^2}{8}$$

$$M_t := M_o + M_d + M_l$$

$$q_u := 1.2 \cdot (q_o + q_d) + 1.6 \cdot q_l$$

$$M_u := \frac{q_u \cdot L^2}{8}$$

$$M_u = 3.914 \times 10^3 \cdot \text{kN} \cdot \text{m}$$

Materiales:

Hormigón:

$$f_c := 35 \cdot \text{MPa}$$

$$f_{cip} := 25 \cdot \text{MPa}$$

$$E_c := 4700 \cdot \sqrt{f_c \cdot \text{MPa}}$$

$$\epsilon_{cu} := 0.003$$

$$\beta_1 := \begin{cases} 0.85 & \text{if } f_c \leq 30 \text{ MPa} \\ 0.85 - 0.008 \cdot \left(\frac{f_c}{\text{MPa}} - 30 \right) & \text{if } 30 \text{ MPa} \leq f_c \leq 55 \text{ MPa} \\ 0.65 & \text{if } f_c \geq 55 \text{ MPa} \end{cases}$$

$$\beta_1 = 0.81$$

Acero para hormigón armado

$$f_y := 420 \text{ MPa}$$

Cables:

$$f_{pu} := 1860 \cdot \text{MPa}$$

$$f_{py} := 1674 \cdot \text{MPa}$$

$$R_s := 0.85$$

$$\epsilon_{pu} := 0.067$$

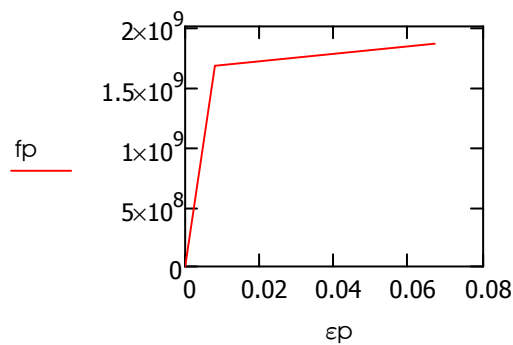
$$\epsilon_{py} := 0.008$$

$$E_p := \frac{f_{py}}{\epsilon_{py}}$$

$$E_p = 2.092 \times 10^5 \cdot \text{MPa}$$

$$\epsilon_p := \begin{pmatrix} 0 \\ \epsilon_{py} \\ \epsilon_{pu} \end{pmatrix}$$

$$f_p := \begin{pmatrix} 0 \\ f_{py} \\ f_{pu} \end{pmatrix}$$



Área tentativa de cable

$$M_n := \frac{M_u}{0.9} \quad M_n = 4.349 \times 10^3 \text{ kN}\cdot\text{m}$$

$$rec := 100 \text{ mm}$$

$$Z := h - rec - \frac{h_{ft}}{2} \quad Z = 1.045 \times 10^3 \text{ mm}$$

$$A_{pt} := \frac{M_n}{0.9 \cdot f_{pu} \cdot Z} \quad A_{pt} = 2.486 \times 10^3 \text{ mm}^2$$

Sección comprimida

$$a := \frac{M_n}{b \cdot \beta_1 \cdot f_c \cdot Z} \quad a = 82.475 \text{ mm}$$

$$Ver1 := \text{if}(a \leq h_{ft}, \text{"viga rectangular"}, \text{"viga T"})$$

$$Ver1 = \text{"viga rectangular"}$$

$$Z_r := h - rec - \frac{a}{2}$$

Excentricidad del cable

$$e := h - c_1 - rec \quad e = 686.789 \text{ mm}$$

Fuerza de pretensado

$$P_e := \frac{(q_o + q_d) \cdot L^2}{8 \cdot e} \quad P_e = 2.303 \times 10^3 \text{ kN}$$

$$P_i := \frac{P_e}{R} \quad P_i = 2.71 \times 10^3 \text{ kN}$$

Calcular área del cable

$$f_{padm} := \min(0.74 \cdot f_{pu}, 0.82 \cdot f_{py}) \quad f_{padm} = 1.373 \times 10^3 \text{ MPa}$$

$$A_p := \frac{P_i}{f_{padm}} \quad A_p = 1.974 \times 10^3 \text{ mm}^2$$

$$A_{pd} := \max(A_{pt}, A_p) \quad A_{pd} = 2.486 \times 10^3 \text{ mm}^2$$

Tensiones admisibles

$$f_{ci} := -0.6 f_{cip} \quad f_{ci} = -15 \cdot \text{MPa}$$

$$f_{ti} := 0.25 \cdot \sqrt{f_{cip} \cdot \text{MPa}} \quad f_{ti} = 1.25 \cdot \text{MPa}$$

$$f_{cs} := -0.6 \cdot f_c$$

$$f_{cs} = -21 \cdot \text{MPa}$$

$$f_{ts} := 0.625 \cdot \sqrt{f_c \cdot \text{MPa}}$$

$$f_{ts} = 3.698 \text{ MPa}$$

Cálculo de tensiones

Estado inicial

$$f_{1i} := \frac{-P_i}{A_c} + P_i \cdot \frac{e}{S_1} - \frac{M_o}{S_1}$$

$$f_{1i} = -1.015 \text{ MPa}$$

$$f_{2i} := \frac{-P_i}{A_c} - P_i \cdot \frac{e}{S_2} + \frac{M_o}{S_2}$$

$$f_{2i} = -9.955 \text{ MPa}$$

Estado de servicio

$$f_{1s} := \frac{-P_e}{A_c} + P_e \cdot \frac{e}{S_1} - \frac{M_t}{S_1}$$

$$f_{1s} = -9.302 \text{ MPa}$$

$$f_{2s} := \frac{-P_e}{A_c} - P_e \cdot \frac{e}{S_2} + \frac{M_t}{S_2}$$

$$f_{2s} = 6.865 \text{ MPa}$$

$$\text{Ver2} := \text{if}(f_{1i} \geq f_{ci}, \text{"Ok"}, \text{"No cumple"})$$

$$\text{Ver2} = \text{"Ok"}$$

$$\text{Ver3} := \text{if}(f_{2i} \geq f_{ci}, \text{"Ok"}, \text{"No cumple"})$$

$$\text{Ver3} = \text{"Ok"}$$

$$\text{Ver4} := \text{if}(f_{1s} \geq f_{cs}, \text{"Ok"}, \text{"No cumple"})$$

$$\text{Ver4} = \text{"Ok"}$$

$$\text{Ver5} := \text{if}(f_{2s} \leq f_{ts}, \text{"Ok"}, \text{"Verificar ancho grieta"})$$

$$\text{Ver5} = \text{"Verificar ancho grieta"}$$

Determinación de armaduras

$$T_u := \frac{M_u}{Z_r \cdot 0.9}$$

$$f_{ps} := 0.9 \cdot f_{pu}$$

$$A_s := \frac{T_u - A_{pd} \cdot f_{ps}}{f_y}$$

$$A_s = -310.138 \text{ mm}^2$$

No requiere armadura a tracción

$$\text{MPa} := 10^6 \cdot \text{Pa}$$

$$\text{kN} := 10^3 \cdot \text{N}$$