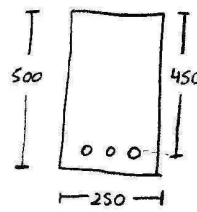


$$\begin{aligned} w_d &= 1,5 \text{ kN/m} \\ w_l &= 5 \text{ kN/m} \\ f'_c &= 25 \text{ [MPa]} \\ f_y &= 420 \text{ [MPa]} \\ \gamma_h &= 25 \text{ kN/m}^3 \end{aligned}$$



Pregunta: Diseñe el refuerzo requerido por la sección para resistir las cargas solicitantes dadas.

→ Peso propio

$$w_o = 25 \cdot 0,5 \cdot 0,25 = 3,125 \text{ [kN/m]}$$

$$w_u = 1,2 (1,5 + 3,125) + 1,6 \cdot 5 = 13,55 \text{ [kN/m]} \rightarrow M_u = w_u \frac{l^2}{8} = 13,55 \cdot \frac{4^2}{8} = 27,1 \text{ [kN-m]}$$

• Diseño de Armadura:

3 Alternativas para cálculo de A_s requerido:

$$i) \alpha = \frac{f_y \cdot A_s}{0,85 f'_c \cdot b} = \frac{420}{0,85 \cdot 25 \cdot 250} \cdot A_s = 0,0791 A_s. \quad M_u = \frac{M_u}{\phi} = f_y \cdot A_s (d - \alpha/2) \rightarrow \frac{27,1 \cdot 10^6}{420 \cdot 0,9} = A_s \left(450 - \frac{0,0791 A_s}{2} \right)$$

↓
Supuesto

$$0,0396 A_s^2 - 450 A_s + 7,169 \cdot 10^4 = 0 \rightarrow A_s = \frac{450 - \sqrt{450^2 - 4 \cdot 0,0396 \cdot 7,169 \cdot 10^4}}{2 \cdot 0,0396} = 161,6 \text{ [mm}^2\text{]}$$

ii) Sea $\alpha \approx 20\% h = 100 \text{ [mm]}$

$$A_s = \frac{M_u}{\phi f_y (d - \alpha/2)} = \frac{27,1 \cdot 10^6}{0,9 \cdot 420 (450 - \frac{100}{2})} = 179,2 \text{ [mm}^2\text{]} \Rightarrow \alpha = \frac{f_y \cdot A_s}{0,85 f'_c \cdot b} = \frac{420 \cdot 179,2}{0,85 \cdot 25 \cdot 250} = 14,17 \text{ [mm]}$$

↓
Supuesto

$$A_s = \frac{27,1 \cdot 10^6}{0,9 \cdot 420 (450 - \frac{14,17}{2})} = 161,9 \text{ [mm}^2\text{]} \Rightarrow \alpha = \frac{420 \cdot 161,9}{0,85 \cdot 25 \cdot 250} = 12,8 \text{ [mm]}$$

$$A_s = \frac{27,1 \cdot 10^6}{0,9 \cdot 420 (450 - \frac{12,8}{2})} = 161,6 \text{ [mm}^2\text{]} \rightarrow \text{Convergió}$$

$$iii) M_u = \frac{M_u}{\phi} = f_y \cdot A_s (\underbrace{d - \alpha/2}_{\approx 0,9d}) \Rightarrow A_s = \frac{M_u}{\phi f_y \cdot 0,9d} = \frac{27,1 \cdot 10^6}{0,9^2 \cdot 420 \cdot 450} = 177 \text{ [mm}^2\text{]}$$

Verifico si: $A_{req} = 161,6 \text{ [mm}^2\text{]}$ comprobando supuesto $\phi = 0,9$

$$\alpha = \frac{f_y \cdot A_s}{0,85 f'_c \cdot b} = \frac{420 \cdot 161,6}{0,85 \cdot 25 \cdot 250} = 12,78 \text{ [mm]} \Rightarrow c = \frac{\alpha}{\beta_1} = \frac{12,78}{0,85} = 15,04 \text{ [mm]}$$

$$\Rightarrow \epsilon_s = \epsilon_u \frac{(d - c)}{c} = 0,003 \left(\frac{450 - 15,04}{15,04} \right) = 0,0868 > 0,005 \rightarrow \phi = 0,9 \text{ OK}$$

Dado $A_s^{req} = 161,6 \text{ [mm}^2\text{]} \rightarrow A_s = 2\phi 12 = 226 \text{ [mm}^2\text{]}. (d = 500 - 40 - 10 - 1\frac{1}{2} = 444.)$

o Verificación de $A_{s,min}$; $A_{s,max}$ ($\epsilon_s \geq 0,004$).

$$A_{s,min} = \max \left\{ \frac{1,4}{f_y} \cdot b_w \cdot d; \frac{\sqrt{f_c'}}{4 f_y} b_w \cdot d \right\} = \left\{ \frac{1,4}{420} \cdot 250 \cdot 444; \frac{\sqrt{25}}{4 \cdot 420} \cdot 250 \cdot 444 \right\} = \{370; 330,4\} \quad (\text{S.10.S.1})$$

$$A_{s,min} = 370 \text{ [mm}^2\text{]}$$

Pero de S.10.5.3: Si $\frac{A_s}{A_s^{req}} \geq 1,33 \Rightarrow$ Cond. como S.10.5.1 y S.10.5.2 no Apl. can.

$$\frac{A_s}{A_s^{req}} = \frac{226}{161,6} = 1,399 \geq 1,33 \Rightarrow A_s = 2\phi 12 = 226 \text{ [mm}^2\text{]}$$

$$A_{s,max}: a = \frac{f_y \cdot A_s}{0,85 \cdot f_c' \cdot b} = \frac{420 \cdot 226}{0,85 \cdot 25 \cdot 250} = 17,87 \text{ [mm.]} \rightarrow c = \frac{a}{\beta_1} = \frac{17,87}{0,85} = 21,02 \text{ [mm.]}$$

$$\epsilon_s = 0,003 \frac{(444 - 21,02)}{21,02} = 0,0604 \geq 0,004 \quad \underline{\text{OK}} \quad A_{s,max}$$

$$\geq 0,005 \Rightarrow \phi = 0,9$$

o Espaciamiento:
$$S = \frac{b - 2 \cdot Rec - 2 \phi_c - n_{barras} \cdot d_b}{n_{barras} - 1} = \frac{250 - 2 \cdot 40 - 2 \cdot 10 - 2 \cdot 12}{1} = 126 \text{ [mm.]}$$

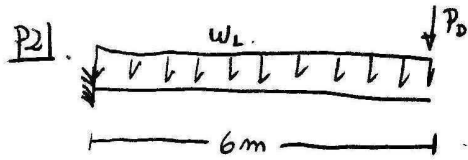
$$S = 126 \text{ [mm]} \geq \begin{cases} d_b > 12 \text{ mm} & \underline{\text{OK}} \\ 25 \text{ mm} & \underline{\text{OK}} \end{cases}$$

o Verificación de $\phi M_n \geq M_u$:
$$\phi M_n = \phi f_y \cdot A_s (d - a/2) = 0,9 \cdot 420 \cdot 226 (444 - \frac{17,87}{2})$$

$$= 37,17 \text{ [kN-m]}$$

$$\phi M_n = 37,17 \text{ [kN-m]} \geq 27,1 \text{ [kN-m]} = M_u //$$

Auxiliar CI 42B (11/09/08).



$$w_L = 12 \text{ [kN/m]}$$

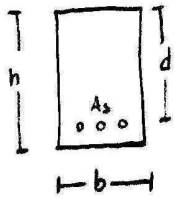
$$P_D = 15 \text{ [kN]}$$

$$f'_c = 30 \text{ [MPa]}$$

$$f_y = 420 \text{ [MPa]}$$

$$\gamma_h = 25 \text{ [kN/m}^3\text{]}$$

Pregunta: Diseñe la viga (dimensiones y refuerzo) para resistir las cargas solicitantes dadas (debe incluirse además el peso propio).



$$M_u = 1,2 \cdot P_D \cdot L + 1,6 \cdot w_L \cdot \frac{L^2}{2} = 1,2 \cdot 15 \cdot 6 + 1,6 \cdot 12 \cdot \frac{6^2}{2} = 453,6 \text{ [kN-m]} \rightarrow \text{Sin considerar peso propio.}$$

o Diseño: Supuesto: $\rho = 0,5 \rho_b \Rightarrow \rho = 0,5 \cdot 0,85 \frac{f'_c}{f_y} \beta_1 \frac{\epsilon_u}{\epsilon_u + \epsilon_y} = 0,5 \cdot 0,85 \cdot \frac{30}{420} \cdot 0,85 \cdot \frac{0,003}{0,003 + 0,0021} = 0,0152$

$$\rho = 0,0152$$

$$a = \frac{f_y \cdot \rho \cdot d}{0,85 \cdot f'_c} = \frac{420 \cdot \rho \cdot d}{0,85 \cdot 30} = 16,47 \cdot \rho \cdot d \quad \wedge \quad \frac{M_u}{\phi} = f_y \cdot \rho \cdot b \cdot d \left(d - \frac{a}{2} \right)$$

$$d = \sqrt{\frac{M_u}{\phi f_y \rho b \left(1 - \frac{16,47 \rho}{2} \right)}}$$

Supuesto: $b = 300 \text{ mm} \Rightarrow d = \sqrt{\frac{453,6 \cdot 10^6}{0,9 \cdot 420 \cdot 0,0152 \cdot 300 \left(1 - \frac{16,47 \cdot 0,0152}{2} \right)}} = \sqrt{\frac{453,6 \cdot 10^6}{1508}} = 548,4 \text{ [mm]}$

Luego, Asumo una sección con $b = 300 \text{ [mm]}$ y $h = 548,4 + 70 = 618,4 \text{ [mm]} \Rightarrow w_o = 25 \cdot 0,3 \cdot 0,6184 = 4,638 \text{ [kN/m]}$

Recalcule M_u considerando peso propio: $M_u = 453,6 + 1,2 \cdot 4,638 \cdot \frac{6^2}{2} = 553,8 \text{ [kN-m]}$

$$d = \sqrt{\frac{553,8 \cdot 10^6}{1508}} = 606 \text{ [mm]} \Rightarrow \text{Asumo } d = 630 \text{ [mm]} \Rightarrow h = 700 \text{ [mm]}$$

$$b = 300 \text{ [mm]}$$

Se puede desde ahora resolver el problema como la P1 o de la forma sigte.

$$A_s = \rho \cdot b \cdot d = 0,0152 \cdot 300 \cdot 630 = 2873 \text{ [mm}^2\text{]}$$

$$\text{Sea } A_s = 3\phi 36 = 3054 \text{ [mm}^2\text{]} \Rightarrow d = 700 - 40 - 10 - \frac{3 \cdot 36}{2} = 632 \text{ [mm]}$$

• Verificación de $A_{s,min}$ y $A_{s,max}$ ($\epsilon_s \geq 0,004$):

$$A_{s,min} = \max \left\{ \frac{1,4}{f_y} b w d; \frac{\sqrt{f_c}}{4 f_y} b w d \right\} = \max \left\{ \frac{1,4}{420} \cdot 300 \cdot 632; \frac{\sqrt{30}}{4 \cdot 420} \cdot 300 \cdot 632 \right\} = \max \{ 632; 612,1 \} = 632 [\text{mm}^2] \quad \checkmark$$

$$A_{s,max} ? \quad a = \frac{f_y \cdot A_s}{0,85 \cdot f'_c \cdot b} = \frac{420 \cdot 3054}{0,85 \cdot 30 \cdot 300} = 167,7 [\text{mm}] \rightarrow c = \frac{a}{\beta_1} = \frac{167,7}{0,85} = 197,3 [\text{mm}]$$

$$\epsilon_s = \epsilon_u \left(\frac{d-c}{c} \right) = 0,003 \left(\frac{632 - 197,3}{197,3} \right) = 0,0066 \geq 0,004 \quad \underline{\text{OK}} (A_{s,max})$$

$$\geq 0,005 \quad (\phi = 0,9)$$

• ESPACIAMIENTO:

$$S = \frac{300 - 2 \cdot 40 - 2 \cdot 10 - 3 \cdot 36}{2} = 46 [\text{mm}] \geq \begin{cases} d_b = 36 [\text{mm}] & \underline{\text{OK}} \\ 25 [\text{mm}] & \underline{\text{OK}} \end{cases}$$

• $\phi M_n \geq M_u$?

$$\phi M_n = \phi f_y \cdot A_s \left(d - \frac{a}{2} \right) = 0,9 \cdot 420 \cdot 3054 \left(632 - \frac{167,7}{2} \right) = 632,8 [\text{KN} \cdot \text{m}]$$

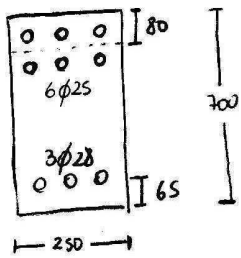
$$M_u = 453,6 + 12 \cdot 25 \cdot 0,3 \cdot 0,7 \cdot \frac{6^2}{2} = 567 [\text{KN} \cdot \text{m}]$$

$$\phi M_n = 632,8 [\text{KN} \cdot \text{m}] \geq 567 [\text{KN} \cdot \text{m}] = M_u \quad \underline{\text{OK}} \quad \left(F.U = \frac{M_u}{\phi M_n} = \frac{567}{632,8} = 0,9 < 1 \quad \underline{\text{OK}} \right)$$

Factor de Utilización

(Nro que sirve como guía de un buen o mal diseño).

P3. Calcule el momento nominal minorado (ϕM_n) para la sección dada para el momento en ambas direcciones (flexión positiva y negativa), considere el caso de ambas armaduras trabajando y en el que solo la armadura traccionada trabaja. Compare resultados.



$$\begin{aligned} f'_c &= 25 \text{ [MPa]} \\ f_y &= 280 \text{ [MPa]} \\ A_{s1} &= 3\phi 28 = 1847 \text{ [mm}^2\text{]} \\ A_{s2} &= 6\phi 25 = 2945 \text{ [mm}^2\text{]} \end{aligned}$$

a) Flexión Positiva:

i) Ambas armaduras: $0,85 f'_c \cdot a \cdot b + f_s' \cdot A_{s2} = f_s \cdot A_{s1}$ Supuestos: $f_s = f_y$ $f_s' \leq f_y$

$$0,85 \cdot f'_c \cdot b \cdot c + E_s \cdot \epsilon_u \frac{(c-d')}{c} \cdot A_{s2} = f_y \cdot A_{s1} \quad * \quad \epsilon_s' = \epsilon_u \frac{(c-d')}{c}$$

$$0,85 \cdot 25 \cdot 250 \cdot 0,85 \cdot c^2 + (200.000 \cdot 0,003 \cdot 2945 - 280 \cdot 1847) c - 200.000 \cdot 0,003 \cdot 2945 \cdot 80 = 0$$

$$4516 c^2 + 1,25 \cdot 10^6 c - 1,414 \cdot 10^8 = 0 \rightarrow c = \frac{-1,25 \cdot 10^6 + \sqrt{(1,25 \cdot 10^6)^2 + 4 \cdot 4516 \cdot 1,414 \cdot 10^8}}{2 \cdot 4516}$$

$$c = 86,25 \text{ [mm]}$$

Luego compruebo supuestos: $\epsilon_s = \epsilon_u \frac{(d-c)}{c} = 0,003 \frac{(700-65-86,25)}{86,25} = 0,0191 \geq 0,0014 = \epsilon_y$ ✓ $(f_y = 280 \text{ [MPa]})$

$$\epsilon_s' = \epsilon_u \frac{(c-d')}{c} = 0,003 \frac{(86,25-80)}{86,25} = 0,0002 < 0,0014 = \epsilon_y \quad \checkmark$$

$$f_s' = E_s \epsilon_s' = 43,48 \text{ [MPa]}$$

$$\phi M_n = \phi (f_y \cdot A_{s1} (d - a/2) + f_s' \cdot A_{s2} (a/2 - d')) = 0,9 (280 \cdot 1847 (635 - \frac{0,85 \cdot 86,25}{2}) + 43,48 \cdot 2945 (\frac{0,85 \cdot 86,25}{2} - 80))$$

$$\phi M_n = 0,9 \cdot 303,9 \text{ [kN-m]} = 273,5 \text{ [kN-m]} \quad \checkmark$$

ii) Solo Armadura traccionada:

$$a = \frac{f_y \cdot A_{s1}}{0,85 f'_c b} = \frac{280 \cdot 1847}{0,85 \cdot 25 \cdot 250} = 97,35 \text{ [mm]} \rightarrow c = 114,5 \text{ [mm]}$$

$$\rightarrow \epsilon_s = 0,003 \frac{(700-65-114,5)}{114,5} = 0,0136 \geq 0,0014 = \epsilon_y \quad (f_s = f_y) \\ \geq 0,005 \quad (\phi = 0,9)$$

$$\phi M_n = \phi f_y \cdot A_{s1} (d - a/2) = 0,9 \cdot 280 \cdot 1847 \cdot (700 - 65 - \frac{97,35}{2}) = 0,9 \cdot 303,2 \text{ [kN-m]} = 272,9 \text{ [kN-m]}$$

b) Flexión Negativa.

i) Ambas Armaduras: $0,85 f_c' \cdot a \cdot b + f_s' \cdot A_{s1} = f_s A_{s2}$ Supuesto: $f_s = f_s' = f_y$

$$a = \frac{f_y (A_{s2} - A_{s1})}{0,85 f_c' \cdot b} = \frac{280 (2945 - 1847)}{0,85 \cdot 25 \cdot 250} = 57,87 \text{ [mm]} \rightarrow c = 68,08 \text{ [mm]}$$

$$\epsilon_s' = 0,003 \left(\frac{68,08 - 65}{68,08} \right) = 0,0001 \leq 0,0014 = \epsilon_y \rightarrow \text{Supuesto } f_s' = f_y \text{ MALO}$$

Supuestos: $f_s = f_y$
 $f_s' < f_y$

$$0,85 f_c' \cdot b \cdot \beta_1 c + E_s \cdot \epsilon_u \frac{(c - d')}{c} \cdot A_{s1} = f_y A_{s2}$$

$$0,85 f_c' \cdot b \cdot \beta_1 c^2 + (E_s \cdot \epsilon_u \cdot A_{s1} - f_y \cdot A_{s2}) c - E_s \cdot \epsilon_u A_{s1} d' = 0$$

$$0,85 \cdot 25 \cdot 250 \cdot 0,85 c^2 + (200.000 \cdot 0,003 \cdot 1847 - 280 \cdot 2945) c - 200.000 \cdot 0,003 \cdot 1847 \cdot 65 = 0$$

$$4516 c^2 + 2,836 \cdot 10^5 c - 7,203 \cdot 10^7 = 0 \rightarrow c = \frac{-2,836 \cdot 10^5 + \sqrt{(2,836 \cdot 10^5)^2 + 4 \cdot 4516 \cdot 7,203 \cdot 10^7}}{2 \cdot 4516}$$

$$c = 98,74 \text{ [mm]} \Rightarrow a = 83,93 \text{ [mm]}$$

Compruebo supuestos: $\epsilon_s' = 0,003 \left(\frac{98,74 - 65}{98,74} \right) = 0,001 \leq 0,0014 = \epsilon_y \text{ OK}$ ($f_s' = 205 \text{ [MPa]}$)

$$\epsilon_s = 0,003 \left(\frac{700 - 80 - 98,74}{98,74} \right) = 0,0158 \geq 0,0014 = \epsilon_y \text{ OK}$$

$$\geq 0,005 \quad (\phi = 0,9)$$

$$\phi M_m = \phi (f_y \cdot A_{s2} (d - a/2) + f_s' \cdot A_{s1} (a/2 - d')) = 0,9 (280 \cdot 2945 (620 - \frac{83,93}{2}) + 205 \cdot 1847 (\frac{83,93}{2} - 65))$$

$$\phi M_m = 0,9 \cdot 467,9 \text{ [kN-m]} = 421,1 \text{ [kN-m]}$$

ii) Solo Armadura traccionada: $a = \frac{f_y \cdot A_{s2}}{0,85 f_c' b} = \frac{280 \cdot 2945}{0,85 \cdot 25 \cdot 250} = 155,2 \text{ [mm]}$ $c = 182,6 \text{ [mm]}$

$$\epsilon_s = 0,003 \left(\frac{620 - 182,6}{182,6} \right) = 0,0072 \geq 0,0014 = \epsilon_y \text{ OK}$$

$$\geq 0,005 \quad (\phi = 0,9)$$

$$\phi M_m = \phi \cdot f_y \cdot A_s (d - a/2) = 0,9 \cdot 280 \cdot 2945 (620 - \frac{155,2}{2}) = 0,9 \cdot 447,3 \text{ [kN-m]} = 402,6 \text{ [kN-m]}$$

Comparación de Resultados:

ϕM_m	Ambas Armaduras	Solo Armadura traccionada	Diferencia (%)
flexión Positiva	273,5 [kN-m]	272,9 [kN-m]	0,22 %
flexión Negativa	421,1 [kN-m]	402,6 [kN-m]	4,4 %