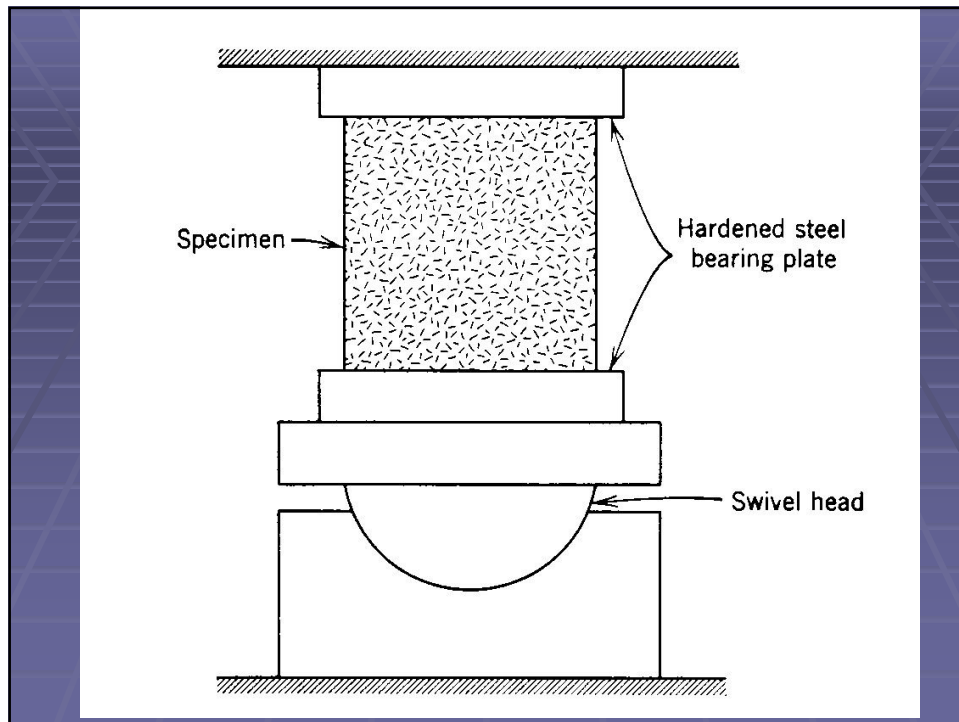


Clase No. 3

- 3.0. Resistencia de la roca intacta
 - 3.1. Concepto de roca intacta y propiedades características.
 - 3.2. Tensores de esfuerzo y deformación.
 - 3.3. Criterios de falla

Ensayo de Compresión Uniaxial





Factores que afectan los resultados

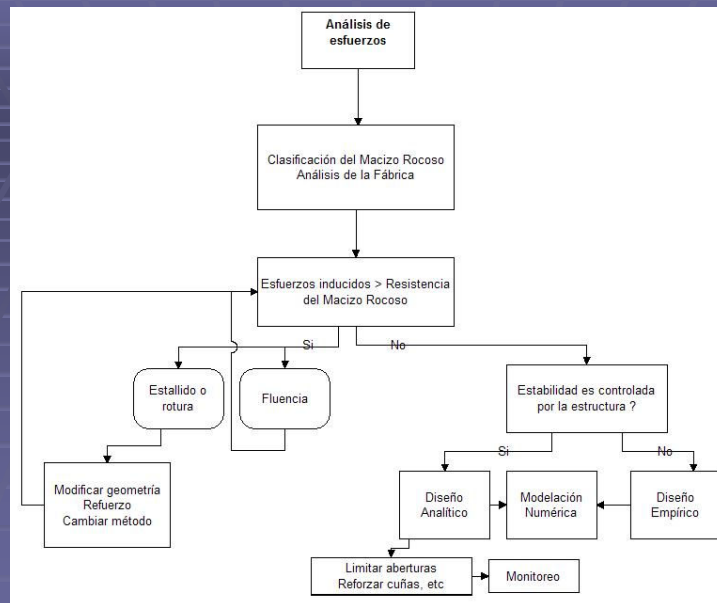
- Caras planas y paralelas de las probetas
- Tamaño de la probeta
- Forma de la probeta
- Contenido de humedad
- Fricción entre probeta y soportes
- Velocidad de carga

Ensayo Triaxial



Criterios de falla

- Falla por Tensión
- Coulomb-Navier
- Mohr Coulomb
- Griffith
- Hoek and Brown

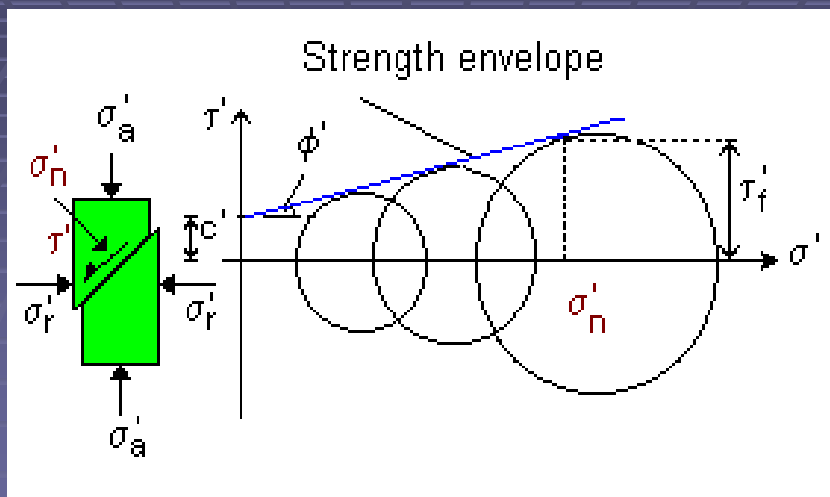


Falla por Tensión

$$\sigma_t = T_0$$

- T_0 : Resistencia a la tracción del macizo rocoso

Mohr Coulomb



Hoek and Brown

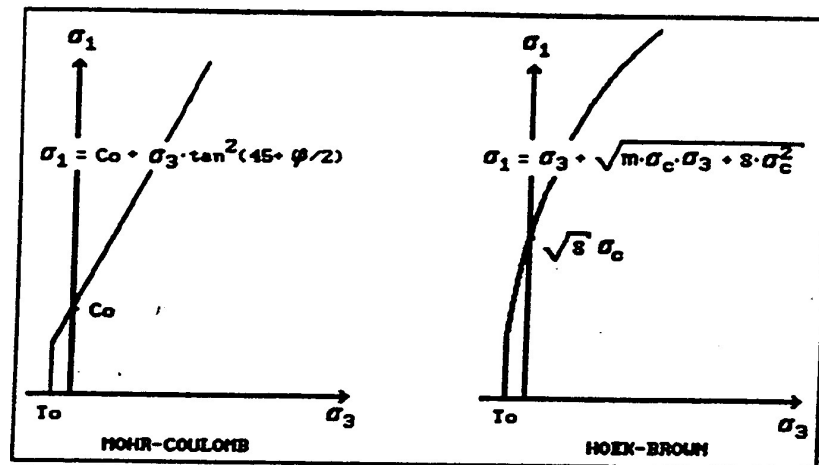
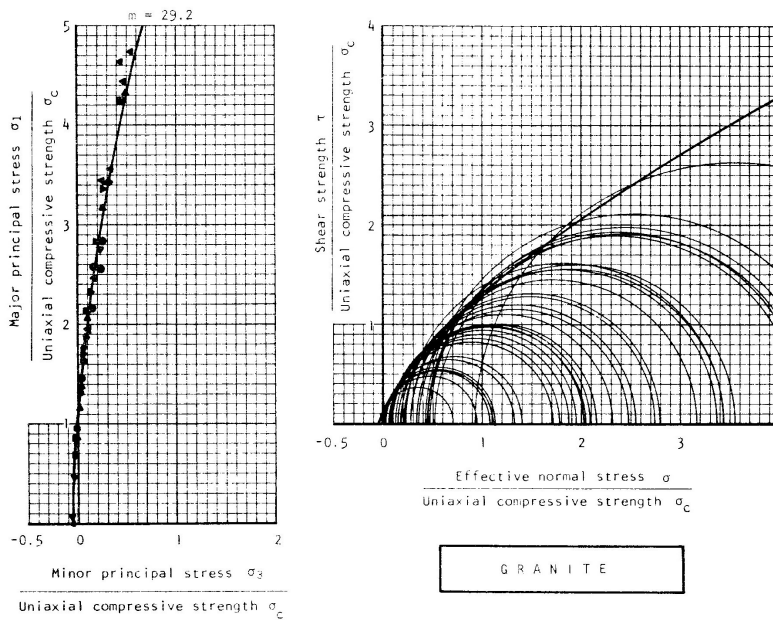
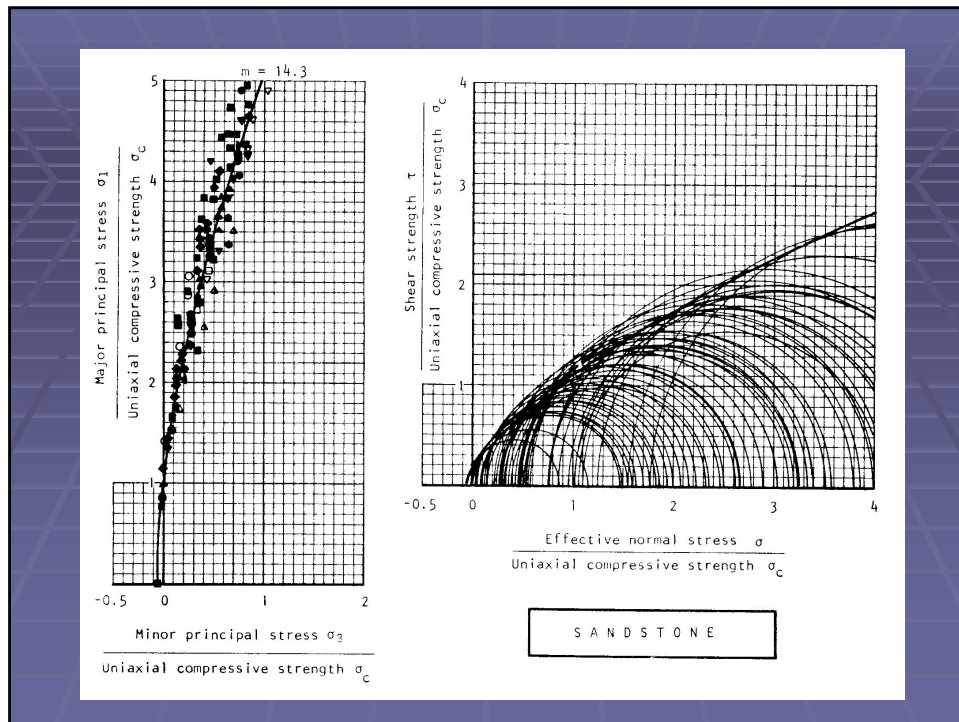


Figure 56 - Failure Criteria





Criterio original Hoek-Brown

$$\sigma_1 = \sigma_3 + \sqrt{m\sigma_3\sigma_c + s\sigma_c^2} \quad (\text{Hoek and Brown, 1980})$$

m, s = constantes que depende de las características del macizo rocoso

σ_c = Resistencia a la compresión simple

Ejemplos: resistencia a la compresión del macizo rocoso,
resistencia a la tracción del macizo rocoso.

APPROXIMATE RELATIONSHIP BETWEEN ROCK MASS QUALITY AND MATERIAL CONSTANTS						
Disturbed Rock Mass m and s Values			Undisturbed Rock Mass m and s Values			
Empirical failure criterion $\sigma_1 = \sigma_3 + (m\sigma_3 + s\sigma_3)^{1/2}$ where σ_1 = major principal stress, σ_3 = minor principal stress, σ_c = uniaxial compressive strength of intact rock, m and s are empirical constants			UNDISTURBED ROCK MASS SINGLE-POINT TEST, EXAMINED JOINTS, SURFACES & DISCONTINUITIES	UNDISTURBED ROCK MASS SINGLE-POINT TEST, EXAMINED JOINTS, SURFACES & DISCONTINUITIES	UNDISTURBED ROCK MASS SINGLE-POINT TEST, EXAMINED JOINTS, SURFACES & DISCONTINUITIES	UNDISTURBED ROCK MASS SINGLE-POINT TEST, EXAMINED JOINTS, SURFACES & DISCONTINUITIES
INTACT ROCK SAMPLES						
Laboratory-size specimens free of discontinuities						
	m	7.00	10.00	15.00	17.00	20.00
RMR = 100	s	1.00	1.00	1.00	1.00	1.00
Q = 500	m	7.00	10.00	15.00	17.00	20.00
	s	1.00	1.00	1.00	1.00	1.00
VERY GOOD QUALITY ROCK MASS						
Tightly interlocking undisturbed rock with unweathered joints at 1-2 m						
	m	2.40	0.43	5.14	5.82	8.55
RMR = 85	s	0.002	0.002	0.002	0.002	0.002
Q = 100	m	4.10	0.55	8.78	9.26	14.50
	s	0.169	0.169	0.169	0.169	0.169
GOOD QUALITY ROCK MASS						
Fresh to slightly weathered rock, slightly disturbed with joints at 1-3 m						
	m	0.575	0.021	1.231	1.355	2.052
RMR = 65	s	0.00003	0.00003	0.00003	0.00003	0.00003
Q = 10	m	2.005	2.825	4.298	4.871	7.163
	s	0.0005	0.0005	0.0005	0.0005	0.0005
FAIR QUALITY ROCK MASS						
Several sets of moderately weathered joints spaced at 0.3-1 m						
	m	0.126	0.163	0.275	0.311	0.456
RMR = 44	s	0.00009	0.00009	0.00009	0.00009	0.00009
Q = 1	m	0.547	1.353	2.003	2.301	3.363
	s	0.00198	0.00198	0.00198	0.00198	0.00198
POOR QUALITY ROCK MASS						
Numerous weathered joints at 30-500 mm with some gouge. Clean compacted waste rock						
	m	0.039	0.041	0.061	0.066	0.102
RMR = 20	s	0.000009	0.000009	0.000009	0.000009	0.000009
Q = 0.1	m	0.447	0.639	0.959	1.027	1.668
	s	0.00019	0.00019	0.00019	0.00019	0.00019
VERY POOR QUALITY ROCK MASS						
Numerous heavily weathered joints spaced at less than 50 mm with gouge. Waste rock with lines						
	m	0.007	0.010	0.015	0.017	0.026
RMR = 5	s	0.0000001	0.0000001	0.0000001	0.0000001	0.0000001
Q = 0.01	m	0.219	0.313	0.469	0.502	0.765
	s	0.00002	0.00002	0.00002	0.00002	0.00002

Criterio modificado Hoek-Brown

$$\sigma_1' = \sigma_3' + \sigma_c \left(m_b \frac{\sigma_3'}{\sigma_c} \right)^a$$

m_b = valor de la constante m para roca fracturada

a = constante para roca quebrada

Criterio de Hoek-Brown generalizado

$$\sigma_1' = \sigma_3' + \sigma_c \left(m_b \frac{\sigma_3'}{\sigma_c} + s \right)^a$$

m_b = valor de la constante m para roca quebrada

a = constante para roca quebrada

$$\sigma_1' = \sigma_3' + \sigma_c \left(m_b \frac{\sigma_3'}{\sigma_c} + s \right)^a$$

where m_b is the value of the constant m for the rock mass
 s and a are constants which depend upon the characteristics of the rock mass
 σ_c is the uniaxial compressive strength of the intact rock pieces and
 σ_1' and σ_3' are the axial and confining effective principal stresses respectively.

For $GSI > 25$ (Undisturbed rock masses)

$$\frac{m_b}{m_i} = \exp \left(\frac{GSI - 100}{28} \right)$$

$$s = \exp \left(\frac{GSI - 100}{9} \right)$$

$$a = 0.5$$

For $GSI < 25$ (Undisturbed rock masses)

$$s = 0$$

$$a = 0.65 - \frac{GSI}{200}$$

$$m_b = m_i \exp\left(\frac{GSI-100}{28-14D}\right)$$

$$s = \exp\left(\frac{GSI-100}{9-3D}\right)$$

$$a = \frac{1}{2} + \frac{1}{6} \left(e^{-GSI/15} - e^{-20/3} \right)$$

(Modificación, 2002)

Table 6.2: Values of the constant m_i after Hoek et al, 1995.

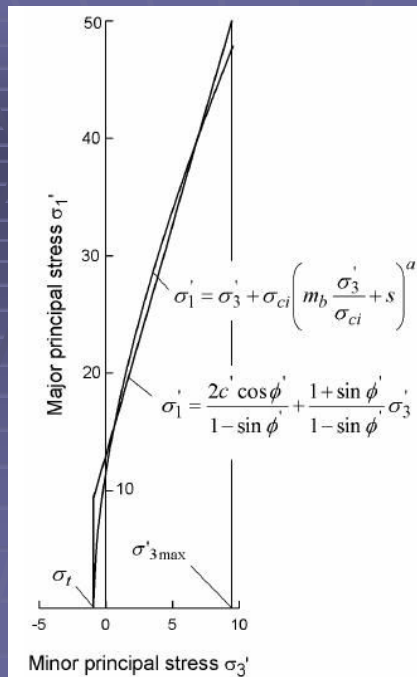
Rock type	Class	Group	Texture				
			Course	Medium	Fine	Very fine	
SEDIMENTARY	Clastic		Conglomerate (22)	Sandstone 19	Siltstone 9	Claystone 4	
			← Greywacke (18) →				
	Non-Clastic	Organic		← Chalk 7 →			
				← Coal (8-21) →			
		Carbonate	Breccia (20)	Spanic Limestone (10)	Micritic Limestone 8		
Chemical		Gypsum 16	Anhydrite 13				
METAMORPHIC	Non Foliated		Marble 9	Hornfels (19)	Quartzite 24		
	Slightly foliated		Migmatite (30)	Amphibolite 31	Mylonites (6)		
	Foliated*		Gneiss 33	Schists (10)	Phyllites (10)	Slate 9	
IGNEOUS	Light		Granite 33		Rhyolite (16)	Obsidian (19)	
			Granodiorite (30)		Dacite (17)		
			Diorite (28)		Andesite 19		
	Dark		Gabbro 27	Dolerite (19)	Basalt (17)		
			Norite 22				
	Extrusive pyroclastic type		Agglomerate (20)	Breccia (18)	Tuff (15)		

Ángulos de fricción y cohesión equivalentes

$$\phi' = \sin^{-1} \left[\frac{6am_b(s + m_b\sigma'_{3n})^{a-1}}{2(1+a)(2+a) + 6am_b(s + m_b\sigma'_{3n})^{a-1}} \right]$$

$$c' = \frac{\sigma_{ci} \left[(1+2a)s + (1-a)m_b\sigma'_{3n} \right] (s + m_b\sigma'_{3n})^{a-1}}{(1+a)(2+a) \sqrt{1 + \left(6am_b(s + m_b\sigma'_{3n})^{a-1} \right) / ((1+a)(2+a))}}$$

where $\sigma_{3n} = \sigma'_{3\max} / \sigma_{ci}$



Ejemplo RocLab

